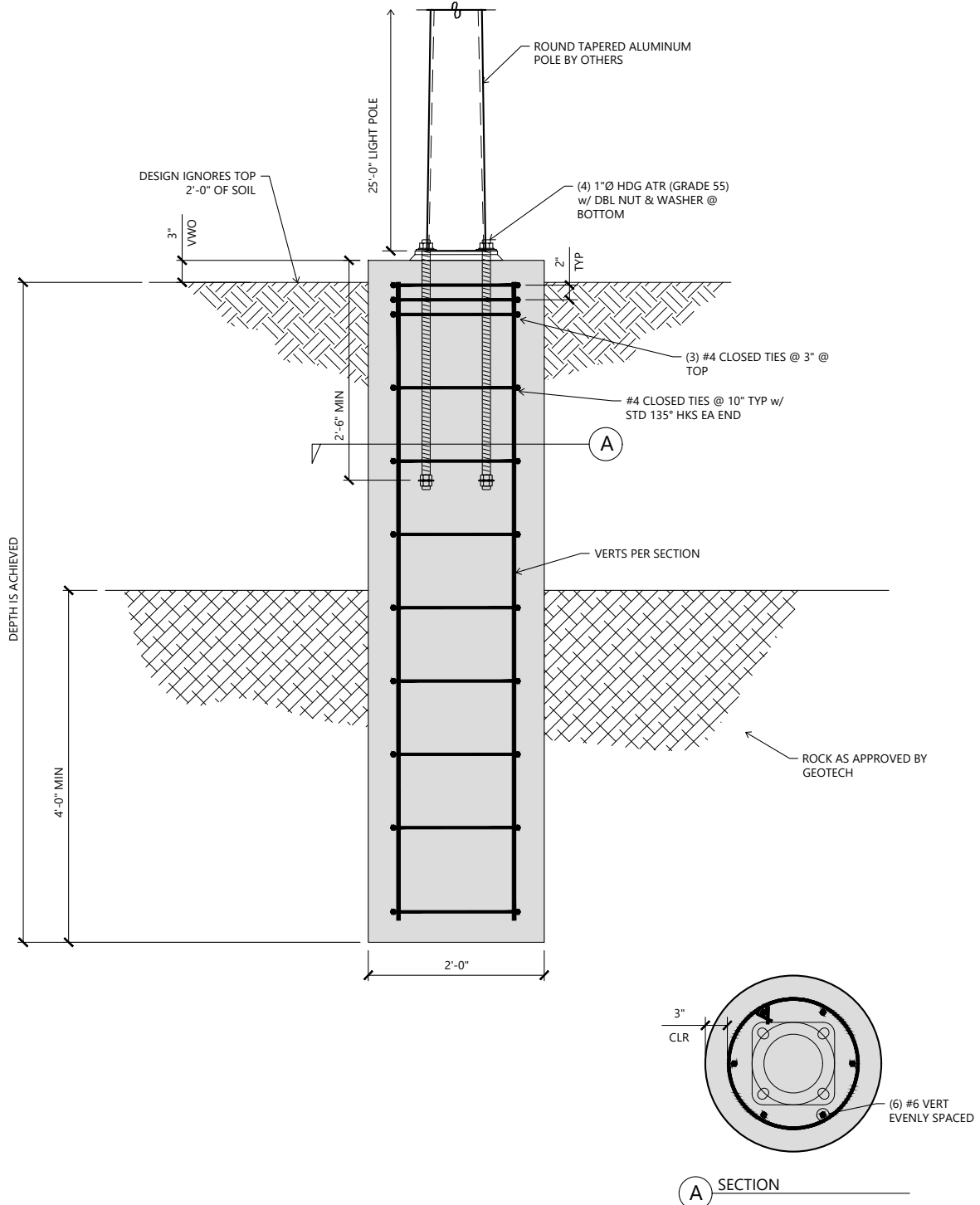


NOTES:
 LIGHT POLE FOUNDATIONS DESIGNED FOR POLE, ARM & LUMINAIRE
 COMBINATIONS:

HAPCO RTA25D7B4M16 w/ AEL AUTOBAN SERIES ATB2 LUMINAIRE

HAPCO RTA25D8B4T1A w/ AEL AUTOBAN SERIES ATB2 LUMINAIRE



Light Pole Foundation Section

STANDARD CURRENT AS OF: 01/01/2025

APPROVED BY: FMD

NOTES:



CAL POLY

LIGHT POLE BASE #1- ALL-THREAD

D11-1

GENERAL STRUCTURAL NOTES

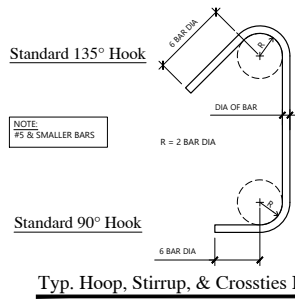
GENERAL

- The Contractor shall verify all dimensions and conditions at the job site prior to starting construction. Notify the Engineer immediately if any discrepancies or inconsistencies are found.
- Details shown on the structural drawings are typical. Similar details apply to similar conditions. In the event certain aspects of the work are not fully detailed on the drawings or specified in these structural notes, the construction shall be of the same character as for similar conditions that are shown or specified and shall be reviewed by the Engineer.
- Existing conditions shown on these drawings are to be field verified by the Contractor as the work progresses. Notify the Engineer immediately if existing conditions are discovered which are different than those shown. Construction shall not proceed until written instructions have been received from the Engineer.
- Notes and details on the drawings shall take precedence over these general notes and typical details. Dimensions shown on the drawings shall take precedence over scale.
- Where there are discrepancies or conflicts between the drawings, notes, and specifications the stricter requirement shall apply unless directed or confirmed with the Architect and Engineer.
- These structural drawings and general notes represent the finished structure. Unless otherwise indicated, they do not specify the method of construction. The Contractor shall provide all measures necessary to protect the structure, workers and other persons during construction. Such measures shall include, but not be limited to, bracing of sufficient strength and stiffness to resist all loads, including wind and seismic loads, imposed on all elements of the structure, shoring for the building, construction equipment, earth banks and retaining walls, forms, scaffolding and planing. These protective measures are the sole responsibility of the Contractor and site visits by the Engineer will not include observation of said measures. Construction materials placed on framed roofs or floors shall be spread out and the resulting loads shall not exceed the design live load for each level.

- Reinforcing materials:
 - ASTM A615 Grade 60.
 - Welded wire fabric: ASTM A1064.
 - Reinforcing for welded inserts: ASTM A706 Grade 60. All bars to be welded shall be marked with a W to designate weldability.
 - Cold drawn spiral reinforcing: ASTM A1064.
- Miscellaneous materials:
 - Sand base under slab on grade: Clean sand with less than 3% passing the #200 sieve and no deleterious material content.
 - Capillary break base under slab on grade: Clean, coarse sand or gravel with 100% passing the 1" sieve, 0-20% passing the #16 sieve, and 0-3% passing the #200 sieve, and no deleterious material content.
 - Vapor retarder: Per Geotechnical Report, 15 mil STEGO W/rap or approved equal.
 - High-strength grout and/or drypack: Non-shrink, non-metallic aggregate type per ASTM C1157. Minimum compressive strength of 7000psi at 28 days when placed in a fluid state. Mix and apply as directed by manufacturer. BASF Masterflow 928 or approved equal.
 - Curing compounds: Water based liquid membranes conforming to ASTM C699 when tested in accordance with ASTM C156: EUCLID Aqua-Cure or approved equal.
 - Expansion joint material: Conform to ASTM D-1751.
 - Joint Sealant: Sikaflex - 1 SL.
- Welding of reinforcing steel: Reinforcing steel shall not be welded unless specifically detailed on the drawings or directed by the Engineer. All welding of reinforcing steel shall be in accordance with the latest edition of "Recommended Practice for Welding Reinforcing Steel, Metal Inserts, and Connections in Reinforced Concrete Construction" by the AWS (AWS D1.4). Use low hydrogen E80x or E90x electrodes for welding of reinforcing steel.
- Lap splices: See Schedule.
- Cover to bars: See Schedule.
- Concrete curing: Keep concrete continuously wet for 7 days or apply curing compound in strict accordance with manufacturer's printed instructions.
- Form removal: Remove forms in accordance with the following schedule:
 - Side forms of footings: Minimum 2 days.
- Vibration: Vibrate all concrete in place with a mechanical vibrator used by experienced personnel.
- Outside diameter of conduit or pipe embedded in slab shall not exceed 30% of slab thickness, or 1-1/2" whichever is smaller, unless specifically detailed otherwise. All conduits or pipes larger than the 1-1/2" or 30% slab thickness (O.D.) shall be placed under the slab. Conduits can be grouped in pairs. Minimum clear distance between single conduits or pairs shall be 2". Minimum clear distance between groups shall be 4". All conduits shall be wrapped in a manner suitable to provide a bond break, and allow concrete expansion/shrinkage.
- Projecting corners of walls, beams, columns, etc. shall be formed with a 3/4" chamfer unless specifically noted otherwise.
- Testing:
 - Laboratory: The Owner shall retain the services of a Testing Laboratory where samples will be tested in accordance with these structural notes and the applicable standards of the ASTM. Work under this division (to be performed by the Contractor) includes the taking and storage of samples and their delivery to the laboratory.
 - Minimum samples: Make a minimum of 3 test cylinders for each day's pour, and as required by ACI 318.
 - Minimum testing of samples: Test cylinders as follows: 1 at 7 days, and 2 at 28 days.
 - Test reports: A copy of all test reports shall be submitted to the Engineer.
- Shop drawings: Submit to the Architect and Engineer for review. No reinforcing shall be placed until reviewed shop drawings have been received on the job. Shop drawings shall consist of both cut and placing sheets. Placing sheets shall contain all information required to position all reinforcing steel without having to refer to the structural drawings.

ABBREVIATIONS

AB	ANCHOR BOLT
AL	ALASKA CEDAR
ADDL	ADDITIONAL
ALUM	ALUMINUM
ALTB	ALTERNATE
ARCH	ARCHITECTURAL
ALL	ALL-THREAD ROD
ALYK	ALASKA-YELLOW CEDAR
BL(K)G	BLOCKING
BM	BEAM
BN	BOTTOM
BOT	DIAPHRAGM BOUNDARY NAILING
BOTTOM	BOTTOM
CA	CALIFORNIA
BTWN	BETWEEN
CA/IF	CALIFORNIA
CANT'L	CANTILEVERED
CFS	COLD FORMED STEEL
CG	CENTER OF GRAVITY
CL	CLEARANCE
CLR	CLEARANCE
CMU	CONCRETE MASONRY UNITS
CONN	CONCRETE CONNECTION
CONT	CONTINUOUS
CONT'D	CENTERED
DBL	DOUBLE
DIA OR Ø	DIAMETER
DECI	DECIMAL
DB	DECK BEAM
DWG	DRAWING
(E)	EXISTING
EL	ELEVATION
ELEV	ELEVATION
EMB'D	EMBEDMENT
EN	SHEARWALL EDGE NAILING
ENVP	ENGINEERED WOOD PRODUCT
EXT	EXTENSION
FB	FLOOR BEAM
FD	FOUNDATION
FLR	FLOOR JOIST
FLR	FLOOR
FN	FIELD NAILING
FRM	FRAMING
FS	FAR SIDE
FTG	FOOTING
GA	GALVANIZED
GB	GRADE BEAM
GLB	GLUE LAMINATED BEAM
HD	HOLLOW
HDR	HOT DIP GALVANIZED
HGR	HANGER
HK	HOOK
HORIZ	HORIZONTAL
INFO	INFORMATION
ING	INGRESS
LONG	LONGITUDINAL
MFR	MANUFACTURER
MAX	MAXIMUM
MACH	MACHINE BOLT
MIN	MINIMUM
(N)	NEW
NOT	NOT TO SCALE
OC	ON CENTER
OP	OPPOSITE HAND
PERF	PERPENDICULAR
P-T	PRE-TENSIONED (BOLT)
PTDR	PRESSURE TREATED DOUGLAS FIR
RF	ROOF BEAM
REFD	REFERENCED
R/S/D	REQUIRED
SCHED	SCHEDULE
SEOR	STRUCTURAL ENGINEER OF RECORD
SHTG	SHATHING
SIM	SMALL
SS	SHEET METAL SCREW
SQ	SQUARE
STN	STAINLESS STEEL
STD	STANDARD
TRM	TRIM
T&B	TOP & BOTTOM
T&G	TONGUE & GROOVE
THK	THICKNESS
TYP	TYPICAL
UNQ	TOP OF WALL
VERT	VERTICAL
VF	VERIFY IN FIELD
VW	VERIFY WITH ARCH
VWC	VERIFY WITH CIVIL
VWG	VERIFY WITH GEOTECH
VWM	VERIFY WITH MICH
VWO	VERIFY WITH OWNER
WTH	WITH
WTS	WELDED THREADED STUD



VERTICAL BAR LAP SPLICE SCHEDULE

BAR SIZE	"L" = LAP SPLICE LENGTH				
	CMU	CONCRETE			
	F _m =1500	F _c =2500	F _c =3000	F _c =4000	F _c =5000
#3	19"	24"	22"	19"	17"
#4	34"	32"	29"	25"	23"
#5	45"	39"	36"	31"	28"
#6	54"	47"	43"	37"	34"
#7	63"	69"	63"	54"	48"
#8	72"	78"	72"	62"	55"
#9	81"	88"	81"	70"	62"
#10	NA	100"	91"	79"	69"
#11	NA	110"	101"	87"	76"

CONCRETE COVER REQUIREMENTS (NON-PRESTRESSED)

COVER FOR REINFORCEMENT SHALL NOT BE LESS THAN THE FOLLOWING:	CONCRETE COVER (IN)
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.	3
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	2
B.1. No. 6 THROUGH No. 18 BARS	1-1/2
B.2. No. 5 BAR, W31 OR D31 WIRE, AND SMALLER	1-1/2
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS:	
C.1. No. 14 AND No. 18 BARS	1-1/2
C.2. No. 11 BAR AND SMALLER	3/4
BEAMS, COLUMNS:	
C.3. PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1-1/2
SHELLS, FOLDED PLATE MEMBERS:	
C.4. No. 8 BAR AND LARGER	3/4
C.5. No. 5 BAR, W31 OR D31 WIRE, AND SMALLER	1/2

FOUNDATION

- Foundation design based on assumed 2022 CBC presumptive bearing values. Actual site conditions may differ from what was assumed. We recommend that the soil conditions are verified by a geotechnical engineer to confirm the assumptions used for design. Without soil verification, the Engineer makes no assurances that settlement (uniform or differential) will not occur.
- Allowable soil bearing values and foundation design are based upon assumed uniform soil conditions. Actual soil conditions which deviate appreciably from that shown or buried structures found during earthwork operations shall be reported to the Architect/Engineer immediately.
- All site work and grading shall be done in accordance with the CBC.
- Excavations for foundations shall conform to the lines and dimensions shown on the drawings. Remove all loose material and debris from excavations and de-water excavations as required to maintain dry working conditions. A Soils Engineer shall approve all site work and foundation excavations prior to installing reinforcing steel or placing concrete.
- A Soils Engineer shall approve all backfill materials prior to placement and observe backfill operations.
- The bottom of all footings shall be horizontal. Where adjoining footings bear at different elevations the bottom of the footings shall be stepped as detailed on the drawings.

CONCRETE

- All concrete work shall be performed in accordance with the latest edition of the ACI Building Code (ACI 318) and the latest edition of the ACI Manuals of Concrete Practice.
- Floor/slab flatness and levelness requirement shall be determined by the architect or owner. Testing as required to verify floor/slab flatness and levelness shall conform to ASTM E1155.
- Submit concrete mix designs to Engineer for review.
- Concrete strength: 3000 psi @ 28 days
- Minimum cement content: 5.7 sacks per yard
- Maximum water-cement ratio: 0.53
- Aggregate size: 1". Provide the maximum ratio of coarse aggregate to fine aggregate consistent with placing requirements.
- Maximum slump: 4-1/2" per ASTM C143.
- Concrete materials:
 - Cement: Portland Type II, ASTM C150.
 - Course aggregate: ASTM C33.
 - Fire aggregate: ASTM C33. Reactivity ratio S_c/R_c shall not exceed one (1) per ASTM C289.
 - Water: potable.
 - Ready mixed concrete: ASTM C94.
 - Water reducing admixture: As required, submit to EOR for review.
 - Air entraining admixture (recommended): Provide between 2% and 4% of entrained air, submit product to EOR for review. Air entraining admixture is recommended for all concrete except hard towed slabs.

STRUCTURAL STEEL AND MISCELLANEOUS METALS

- See Architectural drawings for finishes on steel framing. Steel exposed to weather shall be primed and painted with a high-performance exterior coating approved by the Architect and Engineer or hot-dip galvanized (see below), unless noted otherwise.
- Materials, unless noted otherwise:
 - All other structural steel: ASTM A36.
 - All-thread rod: ASTM F1554.
 - Bolts: ASTM A307.
 - Nuts: ASTM A563A.
 - High strength bolts: ASTM F3125 A325-N.
 - High strength nuts: ASTM A563DN.
 - High strength bolt washers: ASTM F462 Type 1.
 - Welded headed studs: ASTM A108.
 - Stainless steel all thread rods and bolts: ASTM F593 CW1 or CW2 (316).
 - Stainless steel nuts: ASTM F594 CW1 or CW2 (316).
- Installation of high strength bolts: In accordance with AISC "Specification for Structural Joints Using ASTM A325 or A490 Bolts" turn-of-nut method.
- Holes for bolts & threaded rods shall be standard size and nuts/bolts shall be installed Snug Tight per AISC, unless noted otherwise.
- Galvanizing: Steel sections, assemblies, and miscellaneous iron angles, clips, connection hardware, and other elements exposed to weather shall be hot-dip galvanized. Hot-dip galvanized steel framing shall be per ASTM A123 Class 55 minimum. Hot-dip galvanized connection hardware shall be per ASTM A153 or F2329. High strength bolts per ASTM F125 A325 may be mechanically galvanized per ASTM B695 Class 55. Matting bolts and nuts shall receive the same zinc-coating. Repair all uncoated, damaged, or altered galvanized surfaces per ASTM A760. Where field welds are specifically called out, remove zinc completely in the area of the weld. Do not water quench galvanized members and assemblies.
- Shop drawings: Submit to the Architect and Engineer for review. Shop and erection drawings shall contain all information required to install all structural steel without having to refer to the structural drawings. Where member or assemblies are required to be galvanized, shop and erection drawings shall also indicate size and location of drain holes as required per ASTM A585.

DESIGN NOTES

- Dead loads:
 - Light Pole: Self Weight - Per manufacturer
 - Light Fixtures: Self Weight - Per manufacturer
- Wind design data:
 - Occupancy Risk Category = II
 - Basic Wind Speed (S-Cat gust) = 93 mph
 - Wind Exposure(s) = C
- Earthquake design data:
 - Occupancy Risk Category = II
 - Seismic Importance Factor, I = 1.00
 - Site Class = C
 - CSU Spectral Response Coefficients, S_{DS} = 0.86g & S_{DI} = 0.40g
 - Seismic Design Category = D
 - Basic Seismic-Force Resisting System(s) = Nonbuilding Structures
 - Seismic Design Force, C_s = 0.57
 - Seismic Coefficients, R = 1.5
- Design Allowable Values of Soils (1-1/3rd increase for wind and seismic loading assumed):
 - Skin friction = 150 psf
 - Friction coefficient = not using
 - Passive pressure = 500 pc (In Rock), 100 pc (In Soil) acting over 1.5x foundation Diameter

STANDARD CURRENT AS OF: 01/01/2025

APPROVED BY: FMD

NOTES:



CAL POLY

RIGHT POLE BASE #1- ALL-THREAD

D11-2



August 26, 2024

California Polytechnic State University
 1 Grand Avenue, Bldg. 01, Room 129
 San Luis Obispo, CA 93407

Subject: **Proposed Concrete Mixture Design
 Highland Drive Light Pole Bases
 California Polytechnic State University
 San Luis Obispo, California**

Gentlemen:

The attached concrete mixture design is proposed for your use on the above referenced project in accordance with your request. The attached substantiating data indicates that the concrete mixture, when properly batched, sampled, and tested, is capable of providing the indicated strength characteristics. The mixture design is referenced as follows:

<u>MIX NUMBER</u>	<u>USE</u>	<u>DESCRIPTION</u>
611RN1040	Light Pole Bases	6.5 sks; 1" max.; PM ADA W/CM=0.46; 4" Slump 4500 psi @ 28 days

The weights are in pounds per cubic yard of fresh concrete. Aggregate and water weights are for materials in saturated-surface dry condition and must be adjusted for moisture at the time of batching.

The substantiating data for this mixture is derived from similar Mixture No. 564RN1040. The California Building Code allows the use of a similar mixture to qualify a new mixture as long as the data is less restrictive than what is proposed. The primary difference between the two mixtures is that Mixture No. 611RN1040 has a higher cementitious material content. This change will lead to increased compressive strengths at all ages. The following analysis of Mixture Number 564RN1040 is per the California Building Code, Section 1905A. The average compressive strength, 5331 psi, must exceed the following minimum requirements for acceptability.

$$f'_{cr} = f'_c + 1.34ks = 4500 + (1.34)(1.096)(511) = 5250 \text{ psi} < 5331 \text{ psi} \quad \therefore \text{acceptable}$$

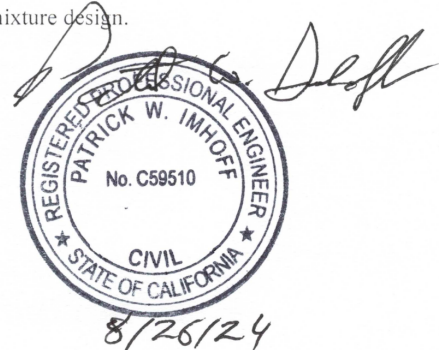
$$f'_{cr} = f'_c + 2.33ks - 500 = 4500 + (2.33)(1.096)(511) - 500 = 5305 \text{ psi} < 5331 \text{ psi} \quad \therefore \text{acceptable}$$

In accordance with ASTM C 94, please provide CalPortland with copies of all results of tests performed on the concrete samples that will be taken on this project.

Please do not hesitate to call if you have questions regarding the proposed mixture design.

Very truly yours,
 CalPortland Co.

Patrick W. Imhoff, P.E.
 Technical Service Manager



Page | 1

P.O. Box 1280 • Santa Maria, CA 93456

Telephone: (805) 345-3400 • Fax: (805) 345-3577

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STANDARD CURRENT AS OF:	01/01/2025
APPROVED BY:	FMD
NOTES:	



LIGHT POLE BASE #1- ALL-THREAD

D11-3



CALPORTLAND®
Concrete Mix Submittal

Submittal Information

Submittal Name Highland Drive Light Pole Bases
Date Submitted 08/26/2024
Customer California Polytechnic State University
Project Name Highland Drive Light Pole Bases

Project Location Cal Poly, SLO
Use Light Pole Bases

Mix Information

Mix ID 611RN1040
Mix Name 4500 psi 1" Crushed
Compressive Strength (f'c) 4500 psi @ 28 Days

Aggregate Nominal Size 3/4"
Air Entrained

Mix Properties

Slump 4"	Sack Content 6.5	94 lb/sack	Total Mass 3921	lb
Air 1.5%	Total Water 33.9	gal	Total Volume 27.00	ft3
W/CM Ratio 0.46	Water/Sack 5.2	gal	Unit Weight 145.2	lb/ft3

Group	Material Description	Supplier	Specific Gravity	Mass lb	Volume ft3
Cement	ADVANCEMENT HS	CalPortland	3.1	519	2.683
Additive	Fly Ash, Class F	ECO Material Technologies	2.4	92	0.614
Water	Water		1	283	4.535
Aggregate	Rocky Canyon 1" x #4 Crushed Granite	CalPortland	2.59	1750	10.828
	Garey Concrete Sand	CalPortland	2.58	1277	7.934
Admixture	Zyla 625 Dosage: 6 fl oz/100 lb CM	GCP Applied Technologies, Inc.	1.12		
Air	Air				0.405

Mix Notes The weights are in pounds per cubic yard of fresh concrete. Aggregate and water weights are for materials in saturated-surface dry condition and must be adjusted for moisture at the time of batching.

Mixes intended for pump placement should be reviewed by the pumping contractor prior to use to ensure compatibility with equipment.

Approval of this mixture design carries the inclusion of CalPortland on the distribution list for all concrete test results. In accordance with ASTM C94, please provide CalPortland with copies of all results of tests performed on the concrete samples that will be taken on this project.

Sincerely, 
Name/Title Patrick Imhoff, P.E. / Technical Services Manager

Contact Patrick Imhoff, P.E.
Phone (805) 345-3472
Email Plmhoff@CalPortland.com

STANDARD CURRENT AS OF: 01/01/2025

APPROVED BY: FMD

NOTES:



CAL POLY

LIGHT POLE BASE #1- ALL-THREAD

D11-4

STATEMENT OF SPECIAL INSPECTIONS, 2022 CBC

Project: _____
 Location: _____

- This Statement of Special Inspections is submitted in fulfillment of the requirements of CBC Sections 1704. Included are:
- Schedule of Special Inspections and tests applicable to this project:
 - Special Inspections per Sections 1704 and 1705
 - Special Inspections for Seismic Resistance
 - Special Inspections for Wind Resistance
 - List of Testing Agencies and other special inspectors that will be retained to conduct the tests and inspections.

Special Inspections and Testing will be performed in accordance with the approved plans and specifications, this statement and CBC Sections 1704 and 1705.

The Schedule of Special Inspections summarizes the Special Inspections and tests required. Special Inspectors will refer to the approved plans and specifications for detailed special inspection requirements. Any additional tests and inspections required by the approved plans and specifications will also be performed.

Interim reports will be submitted to the Building Official and the Registered Design Professional in Responsible Charge in accordance with CBC Section 1704.2.4.

A Final Report of Special Inspections documenting required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy (Section 1704.2.4). The Final Report will document:

- Required special inspections.
- Correction of discrepancies noted in inspections.

The Owner recognizes his or her obligation to ensure that the construction complies with the approved permit documents and to implement this program of special inspections. In partial fulfillment of these obligations, the Owner will retain and directly pay for the Special Inspections as required in CBC Section 1704.2.

This plan has been developed with the understanding that the Building Official will:

- Review and approve the qualifications of the Special Inspectors who will perform the inspections.
- Monitor special inspection activities on the job site to assure that the Special Inspectors are qualified and are performing their duties as called for in this Statement of Special Inspection.
- Review submitted inspection reports.
- Perform inspections as required by the local building code.

SCHEDULE OF INSPECTION, TESTING AGENCIES, AND INSPECTORS

The following are the testing agencies and special inspectors that will be retained to conduct tests and inspection on this project.

Responsibility	Firm	Address, Telephone
1. Special Inspection (except for geotechnical)		
2. Material Testing		
3. Geotechnical Inspections		

SEISMIC REQUIREMENTS (Section 1704.3.2)

Identify seismic-force-resisting system and designated seismic systems subject to special inspections as per Section 1705.11:
 N/A
 The extent of the seismic-force-resisting system is defined in more detail in the construction documents.

WIND REQUIREMENTS (Section 1704.3.3)

Identify main wind-force-resisting system and designated wind resisting components subject to special inspections as per Section 1705.10.
 N/A
 The extent of the main wind-force-resisting system is defined in more detail in the construction documents.

Per CBC 1704.4: Each contractor responsible for the construction of a main wind or seismic force resisting system, designated seismic system or a wind or seismic resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor's statement of responsibility shall contain the following: (1) Acknowledgement of awareness of the special requirements contained in the statement of special inspections. (2) Acknowledgement that control will be exercised to obtain conformance with the construction documents approved by the building official. (3) Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of the reports. (4) Identification and qualification of the person(s) exercising such control and their position(s) in the organization.

SCHEDULE OF SPECIAL INSPECTION

Notation Used in Table:

Column headers:

- C Indicates continuous inspection is required.
- P Indicates periodic inspections are required. The notes and/or contract documents should

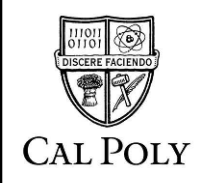
Box entries:

- X Is placed in the appropriate column to denote either "C" continuous or "P" periodic inspections.
- Denotes an activity that is either a one-time activity or one whose frequency is defined in some other manner

Additional detail regarding inspections and tests are provided in the project specifications or notes on the drawings.

Req'd This Job	Verification and Inspection	C	P	Notes
X	1705.3 - Concrete Construction			
X	1. Inspection of reinforcing steel, including prestressing tendons and verify placement		X	
	2. Reinforcing bar welding: a. Verify weldability of reinforcing bars other than ASTM A706; b. Inspect single-pass fillet welds, maximum 3/16"; and c. Inspect all other welds		X X X	
X	3. Inspection of anchors cast in concrete		X	
	4. Inspection of anchors post-installed in hardened concrete members: a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads b. Mechanical anchors and adhesive anchors not defined in 4.a	X	X	Verify with manufacturer
X	5. Verify use of required design mix		X	
X	6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	X		
X	7. Inspect concrete and shotcrete placement for proper application techniques	X		
X	8. Verify maintenance of specified curing temperature and techniques		X	
	9. Inspection of prestressed concrete for: a. Application of prestressing forces; and b. Grouting of bonded prestressing tendons	X X		
	10. Inspect erection of precast concrete members		X	
	11. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs		X	
X	12. Inspect formwork for shape, location and dimensions of the concrete member being formed		X	
X	1705.8 - Required Special Inspections and Tests of Cast-In-Place Deep Foundation Elements			
X	1. Observe drilling operations and maintain complete and accurate record for each element.	X		
X	2. Verify locations of piers and their plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes.	X		
X	3. For concrete elements, perform additional inspections in accordance with CBC Section 1705.3.			

STANDARD CURRENT AS OF:	01/01/2025
APPROVED BY:	FMD
NOTES:	



LIGHT POLE BASE #1- ALL-THREAD

D11-5